

Linear dynamics



Code_Aster, Salome-Meca course material

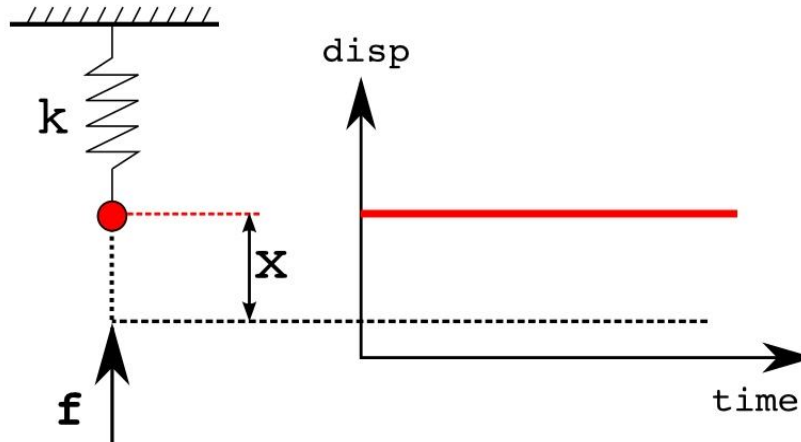
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Outline

- ▶ What is dynamics about ? Inertia !
- ▶ Transient analysis
- ▶ Harmonic analysis
- ▶ Eigen vectors: what are they about ?
- ▶ Model reduction in dynamics
- ▶ Tips
- ▶ Wider scopes
- ▶ Bibliography

What is dynamics about: an illustrative example

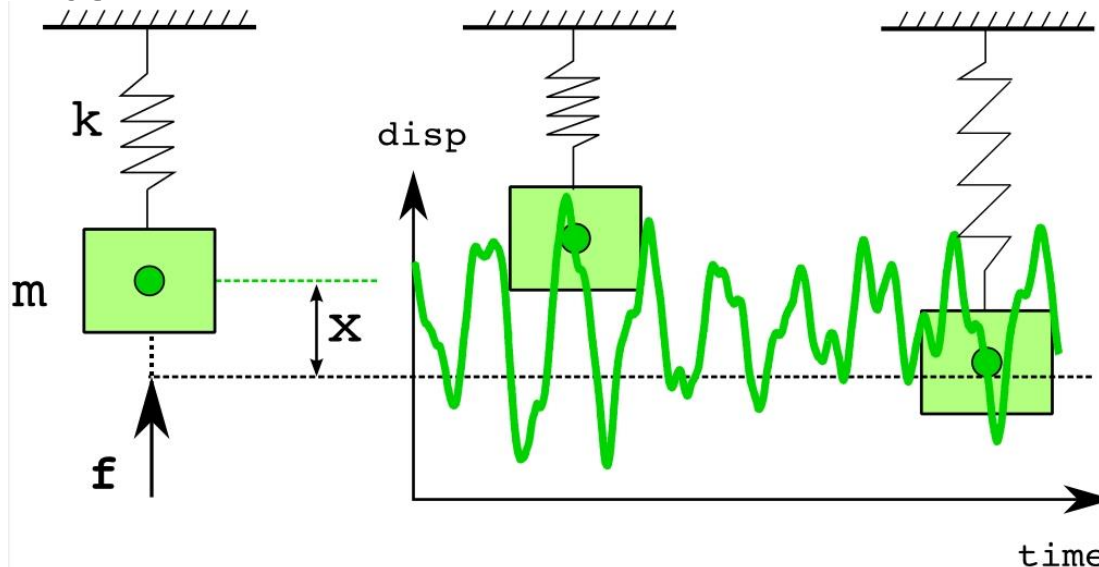
▶ Statics



We seek the system's stationary position

$$k x = f$$

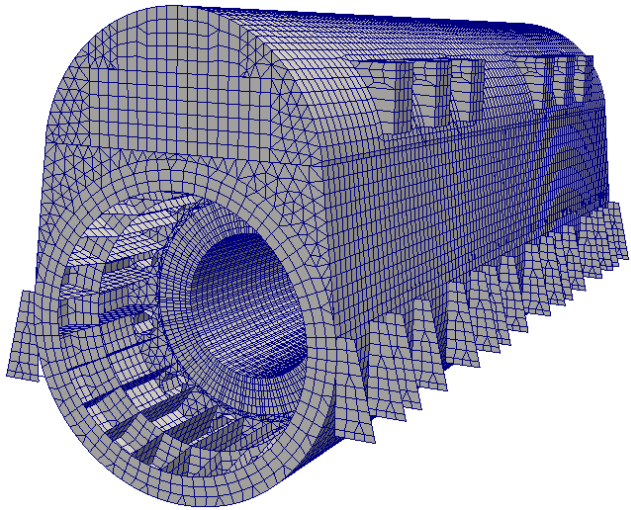
▶ Dynamics



We seek the system's time history

$$m\ddot{x} + kx = f(t)$$

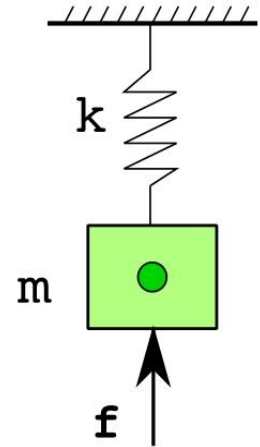
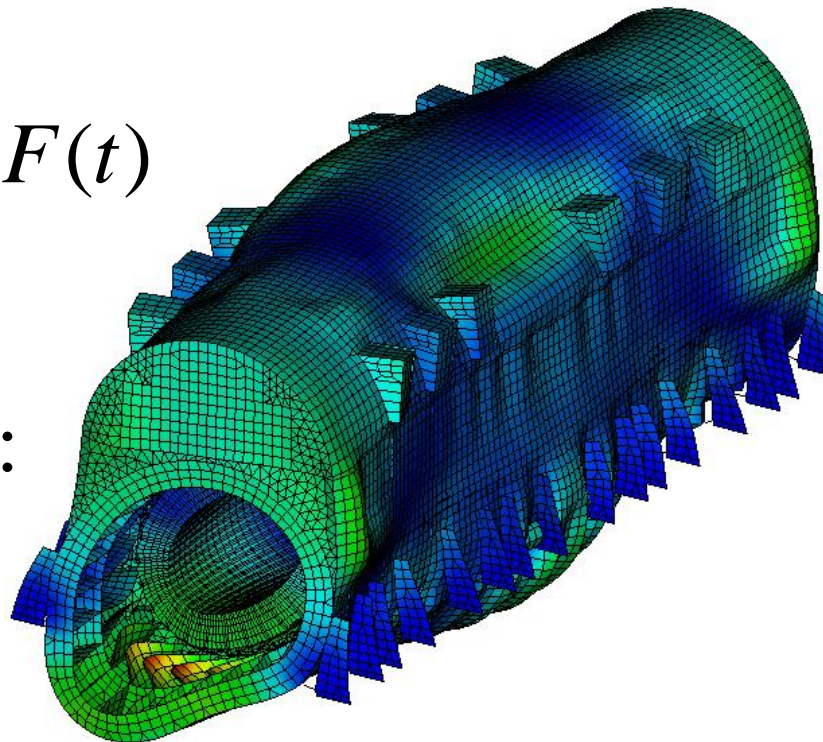
From continuous to discrete : Finite Elements



- Discretization \rightarrow Matrix equation

$$M \ddot{X} + K X = F(t)$$

$$X(t_i) :$$



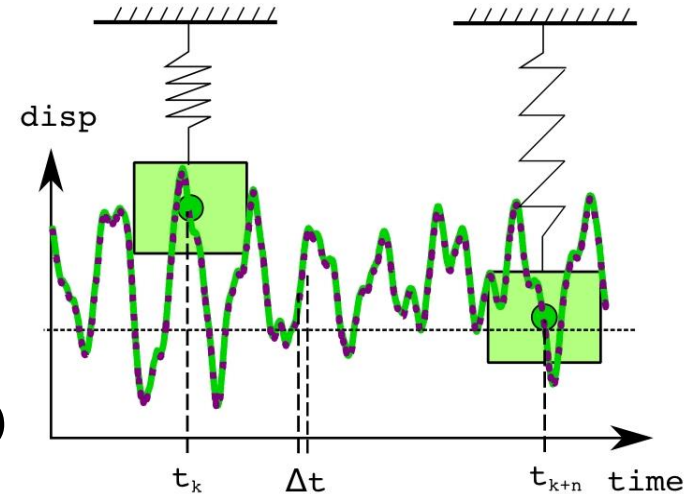
$$m\ddot{x} + kx = f(t)$$

Transient analysis: principles

- ▶ Equation of motion
 - ▶ Separate time and space variables

$$M \ddot{X} + C \dot{X} + K X = F(t) = E u(t)$$

- ▶ We seek the time history $\ddot{X}(t)$; $\dot{X}(t)$; $X(t)$



- ▶ Numerical time integration :

- Force & Inertia balance $\rightarrow \ddot{X}(t_k)$

- Example : first order Euler scheme \rightarrow

$$\begin{cases} \dot{X}(t_{k+1}) = \dot{X}(t_k) + \Delta t \ddot{X}(t_k) \\ X(t_{k+1}) = X(t_k) + \Delta t \dot{X}(t_{k+1}) \end{cases}$$

Transient analysis: implementation

► We need

- Structural matrices M (mass) , C (damping), K (stiffness)
- Excitation field E and its time evolution $u(t)$

► Requirements

- The model → `AFFE_MODELE`
- Materials → `AFFE_MATERIAU`
- Boundary conditions → `AFFE_CHAR_MECA`
- Characteristics of structural elements (if needed) → `AFFE_CARA_ELEM`
- Loads → `AFFE_CHAR_MECA`

► Model assembly

- Matrices M, C , K ; Excitation E → `ASSEMBLAGE`
- Time evolution → `FORMULE / DEFI_FONCTION`

► Resolution with `DYNA_VIBRA` (`TYPE_CALCUL='TRAN'` , `BASE_CALCUL='PHYS'`)

► Post-processing (same tools as in statics)

- `CALC_CHAMP` , `POST_CHAMP` , `POST_ELEM` ...
- Output : `IMPR_RESU`

Harmonic Analysis: principles

- We seek the steady-state response
- Transient analysis for steady-state oscillatory excitation & response

$$M \ddot{X} + C \dot{X} + K X = E u(t)$$

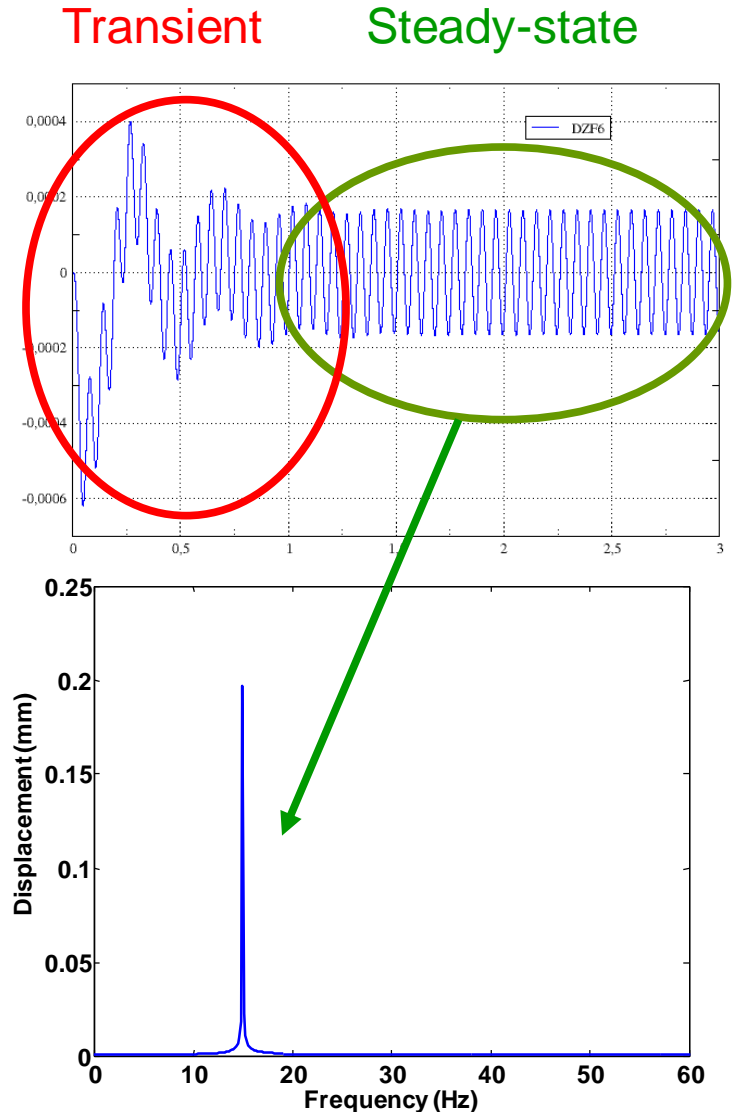
- Alternative approach for steady-state motion

$$u(t) = u_0 e^{j\omega t} \Rightarrow X(t) = X(\omega) e^{j\omega t}$$

↓ (Fourier transform)

$$[-\omega^2 M + j\omega C + K] X(\omega) e^{j\omega t} = E u_0 e^{j\omega t}$$

- Frequency-per-frequency computation



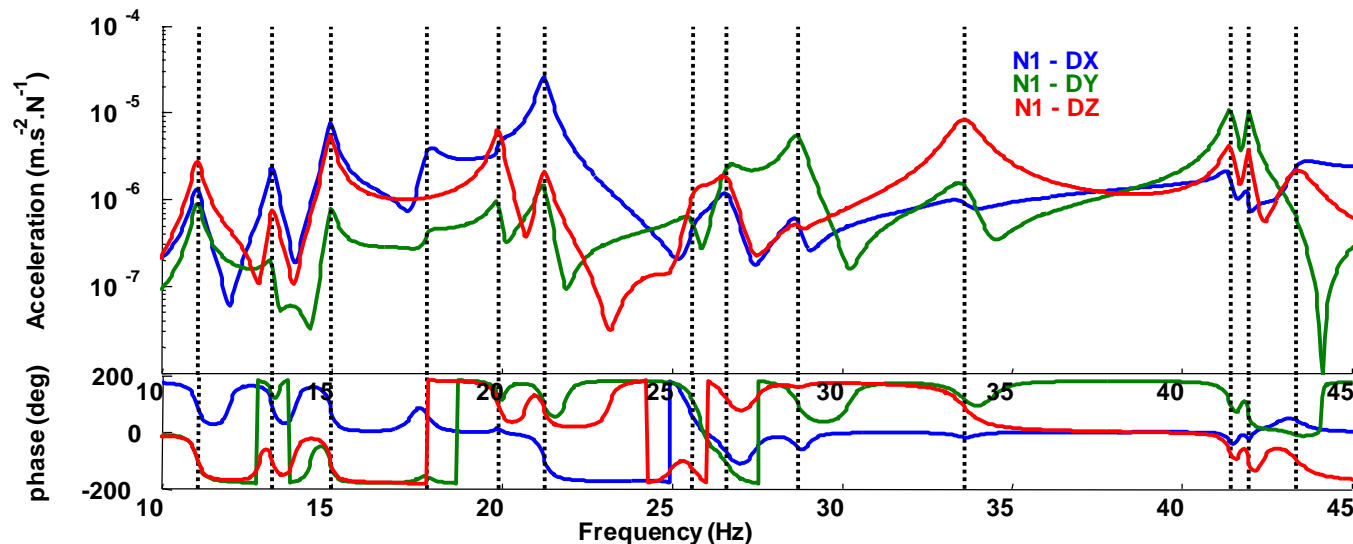
Harmonic analysis: implementation

► We need

- Structural matrices M (mass) , C (damping), K (stiffness)
- Excitation field E and its frequency evolution $u(\omega)$

► Same requirements and model assembly as in transient analysis

► Resolution with `DYNA_VIBRA (TYPE_CALCUL='HARM' ,BASE_CALCUL='PHYS')`



► Post-processing (same tools as in statics)

- `CALC_CHAMP` , `POST_CHAMP` , `POST_ELEM` ...
- Output : `IMPR_RESU`

What are natural frequencies & normal modes ?

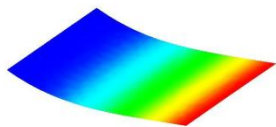
◆ They represent motions where the interchange between the 2 forms of energy (kinetic & potential) can easily occur

◆ Mathematically : $(\phi_k, \omega_k) \mid [-\omega_k^2 M + K] \phi_k = 0 \quad ; \phi_k \neq 0$

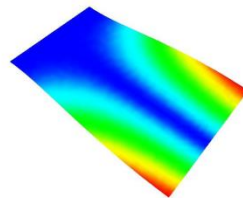
◆ Single DOF : $[-\omega_o^2 m + k] x_o = 0 \quad ; x_o \neq 0 \quad \Rightarrow \quad \omega_o = \sqrt{\frac{k}{m}}$

◆ They depend on the boundary conditions but not on the external loading

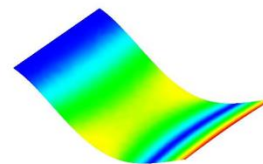
- Natural frequency (or Eigen frequency) : number of oscillations per second
- Normal mode (or Eigen vector) : corresponding deformation shape



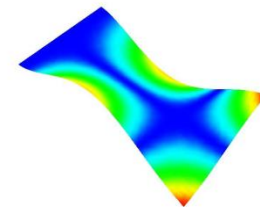
33 Hz



141 Hz



206 Hz

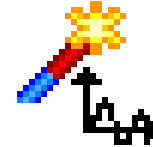


460 Hz

How to get normal modes with Code_Aster ?

▶ The easiest way : the « wizard »

- Graphical user-interface
- Available within SalomeMeca
- Good starting point for transient / harmonic analysis



▶ The usual way : **CALC_MODES**

- M, K assembled matrices are required (+ C matrix if needed)
- Several options are available to refine the computed modes (their number, range, etc.) as well as the algorithm parameters.
- In most cases the default configuration works well

■ **First 10 frequencies**

```
modes = CALC_MODES ( MATR_RIGI= matrigi, MATR_MASS= matmass  
                    OPTION='PLUS_PETITE', CALC_FREQ=_F(NMAX_FREQ= 10) )
```

■ **Frequencies between f1=0.0 Hz and f2=60.0 Hz**

```
modes = CALC_MODES ( MATR_RIGI= matrigi, MATR_MASS= matmass,  
                    OPTION='BANDE', CALC_FREQ=_F(FREQ=(0., 60.)) )
```

Why are normal modes useful ?

- ▶ Modal coordinates provide a “natural” description of the motion

$$X(t) = \phi_1 \eta_1(t) + \dots + \phi_N \eta_N(t)$$

Mode #1 Modal coordinate #1

- ▶ Using modal reduction, the analysis cost is considerably reduced (the number of unknowns, degrees of freedom, is reduced to the number of modes in the considered basis)

$$X(t) \approx \phi_1 \eta_1(t) + \dots + \phi_p \eta_p(t) ; p \ll N$$

- Simple **rule of thumb** : Natural frequencies up to **2 x maximal input frequency**
- Always check the validity of the modal basis (increase the number of modes, static correction)

Model reduction: implementation

- ▶ Same requirements and model assembly as transient or harmonic analysis (M, K, C)
- ▶ Compute normal modes → **CALC_MODES**
- ▶ Model projection (matrices and excitation field) → **PROJ_BASE**
- ▶ Resolution:
 - transient → **DYNA_VIBRA** (TYPE_CALCUL='TRAN', BASE_CALCUL='GENE')
 - harmonic → **DYNA_VIBRA** (TYPE_CALCUL='HARM', BASE_CALCUL='GENE')
- ▶ Back to physical coordinates :
 - whole model → **REST_GENE_PHYS**
 - few points → **POST_GENE_PHYS** (RESU_GENE=...) => table

Damping

▶ Viscous (discrete) damping

- Dashpot modeling using discrete elements

$$M \ddot{X} + C \dot{X} + K X = E u(t)$$

▶ Rayleigh damping $C = \alpha K + \beta M$

- **DEFI_MATERIAU** : **AMOR_ALPHA** & **AMOR_BETA**
- **ASSEMBLAGE** (OPTION = 'AMOR_MECA') → Matrix C

▶ Structural (hysteretic) damping

- Reserved for Frequency Response Analysis $-\omega^2 M \ddot{X} + (j\kappa + 1)K X = Eu(\omega)$
- **ASSEMBLAGE** (OPTION='RIGI_MECA_HYST') → Matrix C

▶ Modal damping

- **DYNA_VIBRA** (TYPE_CALCUL = 'TRAN' , BASE_CALCUL='GENE')

- **AMOR_REDUIT** = 0.01

$$\ddot{\eta}_i + \xi_i \dot{\eta}_i + \omega_i^2 \eta_i = \frac{\phi_i^T E}{m_i} u(t)$$

- **DYNA_VIBRA** (TYPE_CALCUL='TRAN' , BASE_CALCUL='PHYS')

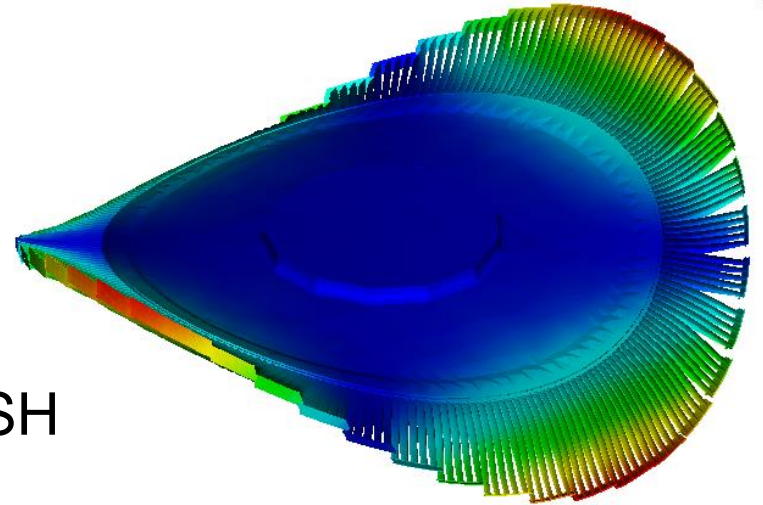
- Can also be used without model reduction but require a modal basis

- **AMOR_MODAL=_F** (MODES=modes , **AMOR_REDUIT** = 0.01)

Printing & visualization of normal modes

▶ Within SALOME_MECA/PARAVIS

- “Macro/modes”
- Magnitudes are arbitrary



▶ Or with other tools : by example GMSH

```
IMPR_RESU(      FORMAT='GMSH', UNITE=37,  
              RESU=_F(RERESULTAT=modes,  
                    NOM_CHAM='DEPL',  
                    TYPE_CHAM='VECT_3D', NOM_CMP=('DX','DY','DZ',),),)
```

▶ Printing frequencies in the .resu file

```
IMPR_RESU(      RESU=_F(RERESULTAT=modes, TOUT_CHAM='NON', NOM_PARA=('FREQ',)))
```

▶ The normal modes are simple displacement fields

Wider scopes

► Limits of modal analysis

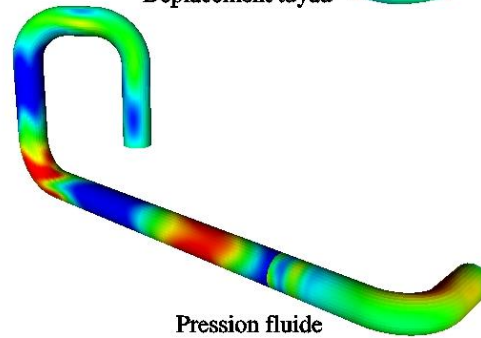
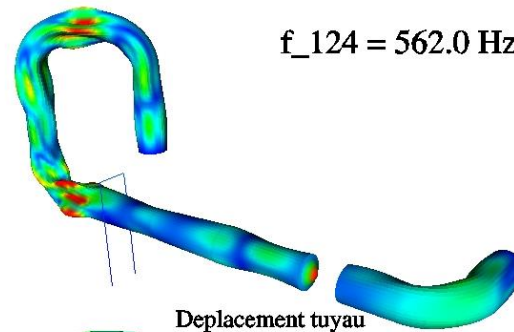
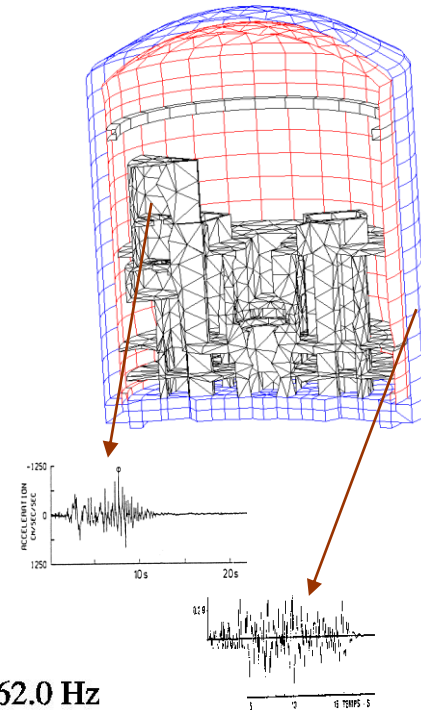
- Linear behaviour only
- With the exception of point contact
(in `DYNA_VIBRA (TYPE_CALCUL='TRAN' ,BASE_CALCUL='GENE')`)

► For non-linear dynamics

- `DYNA_NON_LINE`

► Specific topics

- Fluid-structure interactions
 - Potential flow
 - Acoustic waves
 - Surface waves
- Stochastic
- Seismic analysis
- ...



Tips

▶ EFICAS can help

- Obtain the right syntax (this does not guarantee the rightness of the model !)
- Translation from one version to another (syntax update)

▶ Read U2 & U4 documents

- (and to go further : R for References)

▶ Validation tests are (often) good examples

▶ A modal analysis is **always the starting point**

- Good check of the Finite Elements model (materials, element characteristics, BC, etc.)
- Eigen Frequencies and modes give a good insight on the vibration modes of the structure
- Eigen frequencies can be essential for a suitable choice of integration time step / frequencies for a harmonic analysis

A brief bibliography

▶ <http://www.code-aster.org>

▶ *Mechanical Vibrations - Theory and Application to Structural Dynamics*

M. Géradin, D. Rixen - Wiley

▶ *Vibration Problems in Engineering*

S. Timoshenko - Wiley

▶ *Finite Element Analysis with Error Estimators*

J.E. Akin – Elsevier

▶ *Dynamics of structure*

R.W. Clough, J. Penzien – McGraw-Hill

End of presentation

Is something missing or unclear in this document?
Or feeling happy to have read such a clear tutorial?

Please, we welcome any feedbacks about Code_Aster training materials.
Do not hesitate to share with us your comments on the Code_Aster forum
[dedicated thread](#).

Syntax example : matrix assembly (K,M,...)

Example : 3D model

```
DEBUT()  
#----- model description -----  
ma      = LIRE_MALLAGE ( )  
  
mo      = AFFE_MODELE (MAILLAGE= ma, AFFE = _F(TOUT = 'OUI', PHENOMENE='MECANIQUE',  
                                                MODELISATION=3D))  
  
steel = DEFI_MATERIAU (ELAS = _F(E = 2.1E+11, NU = 0.3, RHO = 7800.)  
cmat   = AFFE_MATERIAU (MAILLAGE=ma, AFFE=_F(TOUT = 'OUI', MATER=steel ))  
  
#----- boundary conditions -----  
block  = AFFE_CHAR_MECA (MODELE=mo,DDL_IMPO=_F(GROUP_MA='BOUND',  
                                                LIAISON='ENCASTRE' )  
  
#----- matrix assembly -----  
ASSEMBLAGE (MODELE= mo, CHARGE= block, CHAM_MATER= cmat,  
            NUME_DDL=CO('nddl'),  
            MATR_ASSE= _F(( MATRICE= CO('matrigi') , OPTION= 'RIGI_MECA' ),  
                          ( MATRICE= CO('matmass') , OPTION= 'MASS_MECA' )))
```

- N.B. : **nddl** is a numbering to assure consistency between matrixes and between vectors

Syntax example : normal modes computation

▶ Computation with `CALC_MODES`

- First 10 frequencies

```
modes = CALC_MODES ( MATR_RIGI= matrigi, MATR_MASS= matmass,  
                    OPTION='PLUS_PETITE', CALC_FREQ=_F(NMAX_FREQ= 10) )
```

- All frequencies between f1=0.0 Hz and f2=100.0 Hz

```
modes = CALC_MODES ( MATR_RIGI= matrigi, MATR_MASS= matmass,  
                    OPTION='BANDE', CALC_FREQ=_F(FREQ= (0.,100.) )
```

▶ Printing modal shapes to SALOME visual interface ('`.med`' format)

```
IMPR_RESU (FORMAT='MED', UNITE=80, RESU=_F (RESULTAT=modes,))
```

▶ Printing the Eigen frequencies in the `.resu` file

```
IMPR_RESU ( RESU=_F (RESULTAT=modes, TOUT_CHAM='NON', NOM_PARA=('FREQ',)))
```

Syntax example: direct time-history analysis

◆ How to use the command

■ Excitation field

- `CHFNO =AFFE_CHAR_MECA(MODELE=MODELE, FORCE_NODALE=_F(GROUP_NO='BOUT', FX=1.0))`

■ Assembly

```
ASSEMBLAGE (MODELE= mo, CHARGE= block, CHAM_MATER= cmat,
            NUME_DDL=CO('nddl'),
            MATR_ASSE= (
                        _F( MATRICE= CO('matrigi') , OPTION= 'RIGI_MECA' ),
                        _F( MATRICE= CO('matmass') , OPTION= 'MASS_MECA' ) ),
            VECT_ASSE= _F( VECTEUR= CO('fx'), CHARGE=CHFNO, OPTION= 'CHAR_MECA' ),)
```

■ Function of time

- Either **FORMULE** : mathematical expression of time (python format)

- NB : time in *Code_Aster* is always noted **'INST'**

- Or **DEFI_FONCTION** : tabulated magnitude

```
impuls = DEFI_FONCTION(NOM_PARA='INST', PROL_DROITE='CONSTANT', PROL_GAUCHE='CONSTANT',
                       VALE=(.0, .0, 0.9, .0, 1.0, g, 2.0, g, 2.1, .0,))
```

■ List of time steps

- `LINST=DEFI_LIST_REEL(DEBUT=0., INTERVALLE=_F(JUSQU_A=tfin, PAS=pa))`

- `CALC_FONC_INTERP` : tabulation of the force on the time steps to optimize the computing time

```
rimpuls = CALC_FONC_INTERP(FONCTION=IMPULS, LIST_PARA=LINST,)
```

■ Transient analysis

- `DLT =`

```
DYNA_VIBRA (
                TYPE_CALCUL='TRAN', BASE_CALCUL='PHYS',
                MATR_MASS=matmass, MATR_RIGI=matrigi,
                SCHEMA_TEMPS=_F(SCHEMA='NEWMARK'),
                EXCIT=_F(VECT_ASSE=fx, FONC_MULT=rimpuls,),
                INCREMENT=_F(LIST_INST=LINST))
```

- How to chose the time step :

- Frequency content of the system
- Frequency content of the input

Syntax example: modal transient analysis

► Projection

```
PROJ_BASE(BASE=modes,  
          MATR_ASSE_GENE=( _F(MATRICE=CO('maspro'), MATR_ASSE=matmass, ),  
                          _F(MATRICE=CO('ripro'), MATR_ASSE=matrigi, ),  
          VECT_ASSE_GENE=( _F(VECTEUR=CO('fxpro'), VECT_ASSE=fx)))
```

► Transient Analysis

```
DTM=DYNA_VIBRA(TYPE_CALCUL='TRAN', BASE_CALCUL='GENE',  
              SCHEMA_TEMPS=_F(SCHEMA='EULER'),  
              MATR_MASS=maspro, MATR_RIGI=ripro,  
              INCREMENT=_F(INST_FIN=tfin, PAS=pa, ),  
              EXCIT=_F(VECT_ASSE_GENE=fxpro,  
                      FONC_MULT=rimpuls))
```

► Back to physical coordinates

- Natural way (whole model)

```
REPHYS=REST_GENE_PHYS(RESU_GENE=DTM, NOM_CHAM=('ACCE', 'DEPL'))
```

can be costly, if the model is huge or the number of steps is significant!

- Efficient way to follow the trajectories or stresses at some points/elements :

```
DXOBS=POST_GENE_PHYS(RESU_GENE=DTM,  
                    OBSERVATION=_F(NOM_CHAM='DEPL',  
                                    NOM_CMP='DX',  
                                    GROUP_NO='OBS'))
```