

## SSNA104 - Hollow roll subjected to a pressure, linear viscoelasticity

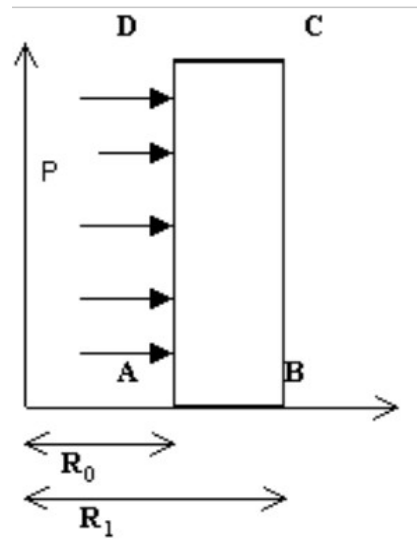
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### Summary:

This CAS-test makes it possible to validate the laws of `LEMAITRE` and `LEMA_SEUIL` established in `Code_Aster` in the case of linear viscoelastic behavior. The found results are compared with an analytical solution.

## 1 Problem of reference

### 1.1 Geometry



Dimensions of the cylinder:

$$R_0 \quad 1 \text{ m}$$

$$R_1 \quad 2 \text{ m}$$

Figure 1.1-a: Cut of the hollow roll and loading

### 1.2 Properties of materials

Young modulus:  $E = 1 \text{ MPa}$

Poisson's ratio:  $\nu = 0.3$

Law of LEMAITRE :

$$g(\sigma, \lambda, T) = \left( \frac{1}{K} \frac{\sigma}{\lambda^m} \right)^n \quad \text{with} \quad \frac{1}{K} = 1, \quad \frac{1}{m} = 0, \quad n = 1$$

Law LEMA\_SEUIL :

$$g(\sigma, \lambda, T) = A \left( \frac{2}{\sqrt{3}} \sigma \right) \Phi \quad \text{with} \quad A = \frac{\sqrt{3}}{2}, \quad \Phi = 1 \quad \text{on all the grid}$$

$$S = 10^{-10}$$

being given the value of the various parameters materials, the two laws are absolutely identical and can thus be compared with the same analytical solution.

## 1.3 Boundary conditions and loading

### Boundary conditions:

The cylinder is blocked in  $DY$  on the sides  $[AB]$  and  $[CD]$ .

### Loading:

The cylinder is subjected to a pressure interns on  $[DA]$   $P0 = 1.E - 3 MPa$

## 2 Reference solutions

### 2.1 Method of calculating used for the reference solutions

The whole of this demonstration can be read with more details in the document [bib1].

In the case of a linear viscoelastic isotropic material, one can describe the behavior in the course of time using two functions  $I(t)$  and  $K(t)$  so that strains and stresses can be written:

$$\varepsilon(t) = (I + K) * \frac{d\sigma(t)}{d\tau} - K * \frac{d(\text{Tr}(\sigma(t)))}{d\tau} I_3$$

where  $I_3$  indicate the matrix identity of row 3

and  $*$  the product of convolution:  $(f * g)(t) = \int_0^t f(t-\tau) g(\tau) d\tau$

One finds  $I(t) = \frac{1}{E} + kt$ ,  $K(t) = \frac{\nu}{E} + \frac{1}{2}kt$

The pressure is imposed  $P_0$  at the moment  $t=0$ , the internal pressure is worth  $p(t) = H(t) P_0$

where  $H(t) = \begin{cases} 0 & \text{si } t-\tau < 0 \\ 1 & \text{si } t-\tau \geq 0 \end{cases}$  with in this case  $\tau = 0$

One uses the transform of Laplace Carson  $f^+(n) = L(f(t)) = n \int_0^\infty f(t) e^{-nt} dt$

From where  $p^+ = P_0$

The solution of the elastic problem are equivalent is:

$$\sigma^+ = \begin{pmatrix} \gamma \left(1 - \frac{r_1^2}{r^2}\right) & 0 & 0 \\ 0 & \gamma \left(1 + \frac{r_1^2}{r^2}\right) & 0 \\ 0 & 0 & \sigma_z^+ \end{pmatrix} \quad \text{where } \gamma = \frac{P_0 r_0^2}{r_1^2 - r_0^2}$$

One determines  $\sigma_z^+$  by the condition on  $\varepsilon_z^+$  data by the boundary conditions:

$$\varepsilon_z^+ = 0 = (I^+ + K^+) \sigma_z^+ - K^+ (2\gamma + \sigma_z^+) = I^+ \sigma_z^+ - 2K^+ \gamma$$

From where  $\sigma_z^+ = \gamma \left(1 + \frac{(2\nu - 1)p}{p + Ek}\right)$ .

One finds by the transform of opposite Laplace  $\sigma_z(t) = \gamma(1 - (1 - 2\nu)e^{-Eht})$ , in the same way by applying the transform of Laplace reverses on  $\sigma_r$  and  $\sigma_\theta$ , one finds

$$\sigma^+ = \begin{pmatrix} \gamma \left(1 - \frac{r_1^2}{r^2}\right) & 0 & 0 \\ 0 & \gamma \left(1 + \frac{r_1^2}{r^2}\right) & 0 \\ 0 & 0 & \gamma(1 - (1 - 2\nu)e^{-Eht}) \end{pmatrix}$$

One from of deduced:

$$\dot{\varepsilon}_V = \begin{pmatrix} \frac{3}{2}k\gamma \left(\frac{1-2\nu}{3}e^{-Ekt} - \frac{r_1^2}{r^2}\right) & 0 & 0 \\ 0 & \frac{3}{2}k\gamma \left(\frac{1-2\nu}{3}e^{-Ekt} - \frac{r_1^2}{r^2}\right) & 0 \\ 0 & 0 & -k\gamma((1-2\nu)e^{-Eht}) \end{pmatrix}$$

and while integrating with  $\varepsilon_V(0) = 0$  ;

$$\varepsilon_V = \begin{pmatrix} \frac{3}{2}\gamma \left(\frac{1-2\nu}{3}e^{-Ekt} - k\frac{r_1^2}{r^2}t\right) & 0 & 0 \\ 0 & \frac{3}{2}\gamma \left(\frac{1-2\nu}{3}e^{-Ekt} - k\frac{r_1^2}{r^2}t\right) & 0 \\ 0 & 0 & -\gamma \frac{(1-2\nu)}{E}(1 - e^{-Eht}) \end{pmatrix}.$$

One from of deduced radial displacement

$$w(r, t) = r\gamma \left[ \frac{1}{E} \left[ (1 + \nu) \frac{r_1^2}{r^2} + \frac{1-2\nu}{2} (3 - (1-2\nu)e^{-Ekt}) \right] + \frac{3}{2}k \frac{r_1^2}{r^2}t \right]$$

## 2.2 Results of reference

Displacement  $DX$  on the node  $B$  and constraints  $SIXX$ ,  $SIYY$  and  $SIZZ$  in  $B$

## 2.3 Uncertainty on the solution

0% : analytical solution

## 2.4 Bibliographical references

PH. BONNIERES: Two analytical solutions of axisymmetric problems in linear viscoelasticity and with unilateral contact, Note HI-71/8301

## 3 Modeling A

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### 3.1 Characteristics of modeling

The problem is modelled in axisymetry.

### 3.2 Characteristics of the grid

1000 meshes QUAD4

### 3.3 Sizes tested and results

| Identification | Moments | Reference   |
|----------------|---------|-------------|
| $DX(B)$        | 0.9     | 2.14498 E-3 |
| $SIXX(B)$      | 0.9     | 0.0         |
| $SIYY(B)$      | 0.9     | 2.7912 E-4  |
| $SIZZ(B)$      | 0.9     | 6.66 E-4    |

## 4 Modeling B

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### 4.1 Characteristics of modeling

The problem is modelled in axisymetry

### 4.2 Characteristics of the grid

1000 meshes QUAD4

### 4.3 Sizes tested and results

| Identification | Moments | Reference   |
|----------------|---------|-------------|
| $DX(B)$        | 0.9     | 2.14498 E-3 |
| $SIXX(B)$      | 0.9     | 0.0         |
| $SIYY(B)$      | 0.9     | 2.7912 E-4  |
| $SIZZ(B)$      | 0.9     | 6.66 E-4    |

## 5 Summary of the results

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Results calculated by *Code\_Aster* are in agreement with the analytical solutions but very strongly depend on the refinement of the grid.