

## **SDLX101 - Checking of chaining MISS3D -Code\_Aster in the case of a building of big size**

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### **Summary:**

This test of nonregression implements a calculation of interaction ground-structure carried out thanks to the chaining *Code\_Aster* - MISS3D by the frequential method of coupling on a model 3D of building resting on a homogeneous ground.

Spectra of answer obtained starting from the model 3D of this building and using the chain *Code\_Aster* - MISS3D constitute a first result of reference.

One validates by comparison its modes of `CALC_MISS/TYPE_RESU 'TRAN_GENE'` and `'HARM_GENE'`, for the loadings by signal in acceleration and displacement, defined in time or frequency.

## 1 Problem of reference

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### 1.1 Geometry

Software MISS3D uses the frequential method of coupling to take account of the interaction ground - structure. This method, based on of dynamic under-structuring, consists in cutting out the field of study in three under-fields:

- ground,
- the foundation,
- the building.

The geometry taken into account is that of a building extended on erasing single.

#### Geometry of the model of the foundation

The foundation raft is cruciform, as shows it below it [Figure 1.1-a]:

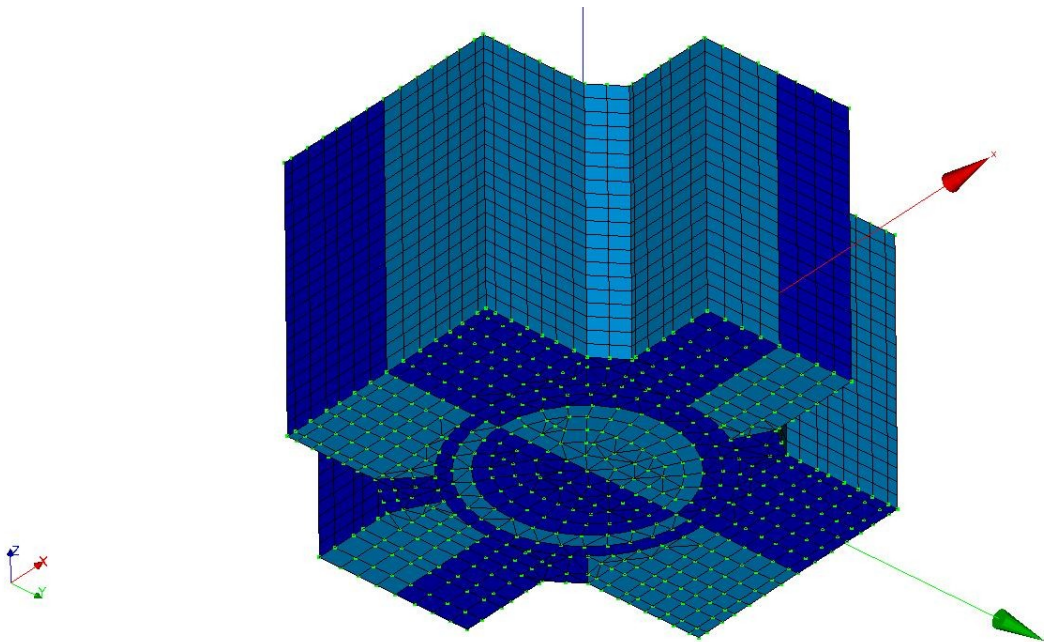


Figure 1.1-a: Foundation of the building

## 1.2 Properties of materials

An average homogeneous ground is considered whose characteristics are summarized in table Ci - afterwards:

| Sleep      | Thickness (m) | $\rho$ (kg/m <sup>3</sup> ) | $\nu$ | $E$ (MPa) | $\beta$ |
|------------|---------------|-----------------------------|-------|-----------|---------|
| Lay down 1 | 113.4         | 2100                        | 0.4   | 7000      | 0.1     |

**Table 1.2-1: Soil mechanics characteristics homogeneous**

The foundation and the building are out of reinforced concretes, prestressed and rigid:

| Material    | $E$ (Pa) | $\rho$ (kg/m <sup>2</sup> ) | $\nu$ |
|-------------|----------|-----------------------------|-------|
| armed       | 3.5 E10  | 0 ⇒ 12500                   | 0.2   |
| prestressed | 4.0 E10  | 2.5 E3                      | 0.2   |
| rigid       | 4.0 E11  | 0.                          | 0.2   |

## 1.3 Boundary conditions and loadings mechanical

The foundation is regarded as rigid. This condition is ensured by a solid connection on the group of meshes of the foundation.

The seismic excitation of the structure is carried out by applying 3 accélérogrammes such as:

| Direction | Accélérogramme | Normalizes |
|-----------|----------------|------------|
| $X$       | acc1.c2        | 0.1 g      |
| $Y$       | acc2.c2        | 0.1 g      |
| $Z$       | acc3.c2        | 0.06 g     |

**Table 1.3-1: Seismic excitations**

One thus gives here the accélérogramme (normalized to 0.1g ) and the spectrum of each elementary excitation for a damping of 4%.

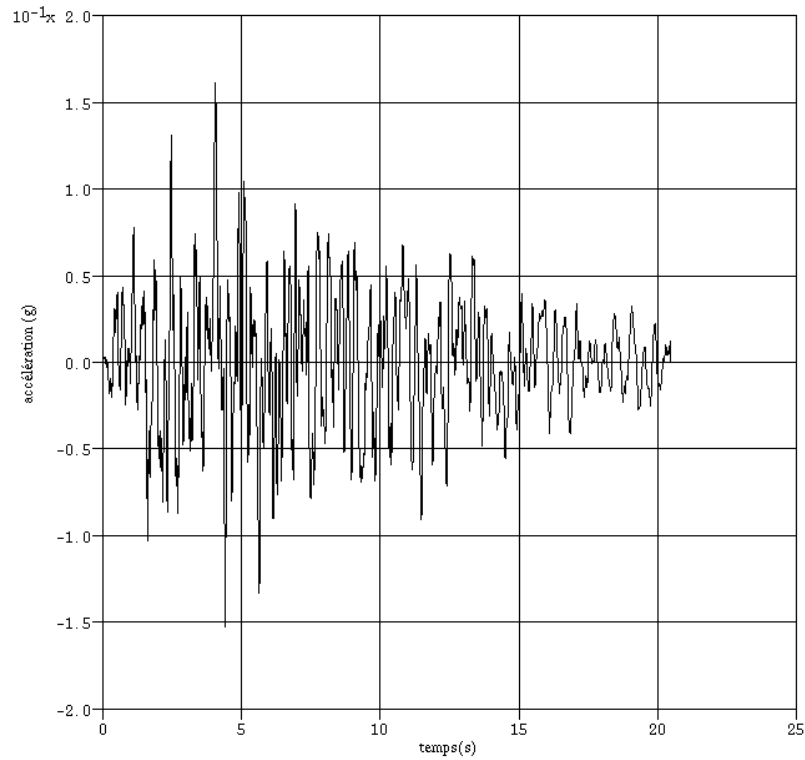


Figure 1.3-a: Accélérogramme acc1.c2

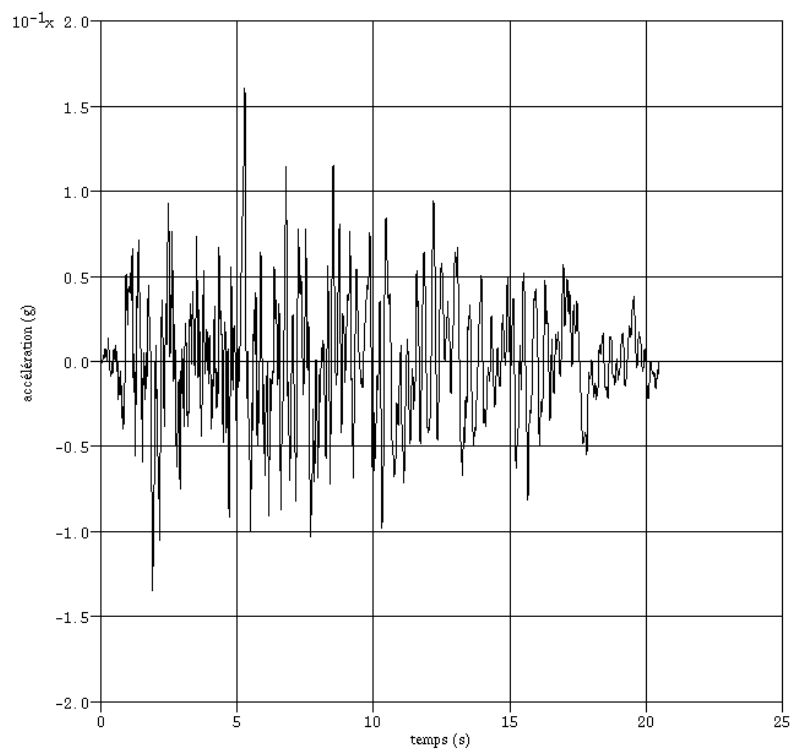


Figure 1.3-b: Accélérogramme acc2.c2

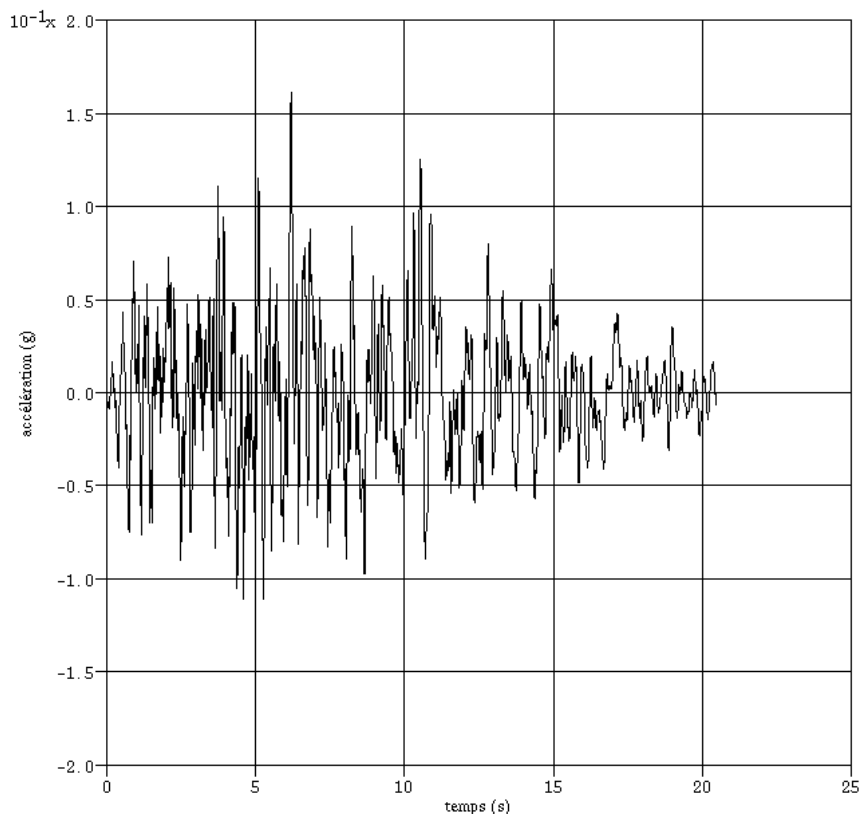


Figure 1.3-c: Accélérogramme acc3 . c2

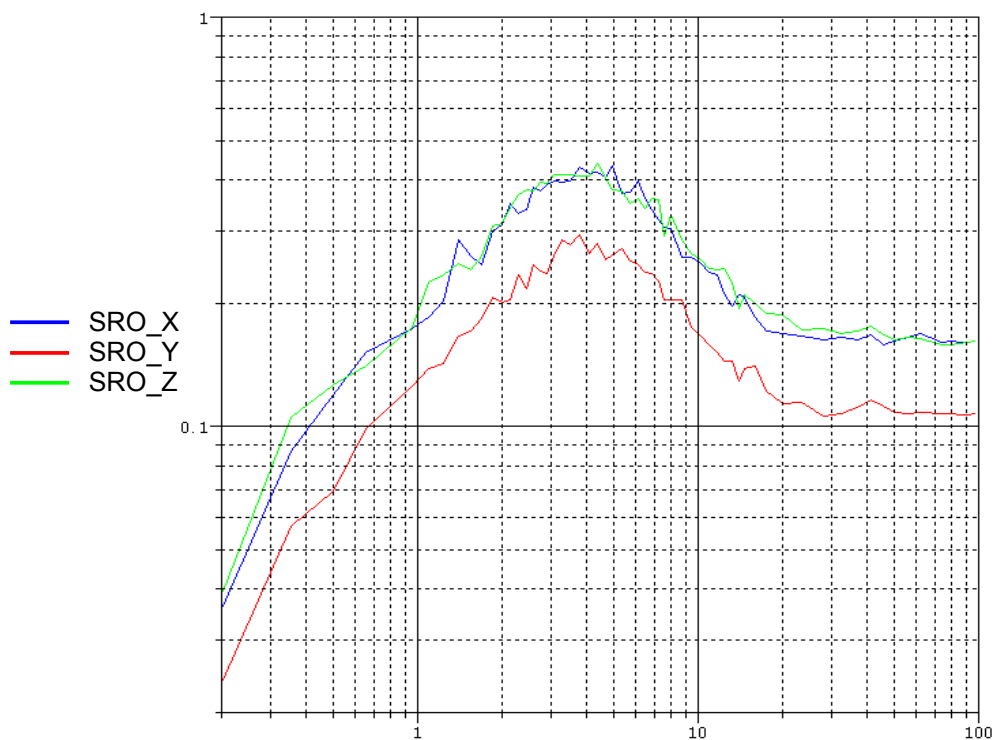


Figure 1.3-d: Spectra of the excitation

A second calculation is done by providing accélérogrammes on frequential basis directly. The results are then compared with the first calculation.

One also checks by direct comparison its modeS of `CALC_MISS/TYPE_RESU 'TRAN_GENE'` and `'HARM_GENE'`, for the loadings by signal in acceleration and displacement. The comparison being done on spectra of oscillator, one calculates previously the FFT reverses harmonic result with the operator `REST_SPEC_TEMP`.

## 2 Reference solution

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### 2.1 Method of calculating used for the reference solution

Case test of nonregression for the first calculation (accélérogrammes on temporal basis), reference 'AUTRE\_ASTER' for the second calculation (accélérogrammes on frequential basis) with the first calculation for reference.

### 2.2 Results of reference

The spectra of floor were calculated in pseudo-acceleration with a damping of 4% in the horizontal directions  $X$  and  $Y$  at the top of the building.

One calculates the spectra on certain nodes of the top of the grid. The final spectrum is obtained by taking the envelope of the directions  $X$  and  $Y$ .

### 2.3 Bibliographical reference

- 1) G. QUILTON: "Presentation and examples of use of CLASSI: Computer code of analysis of the effects of the interaction ground - structure on the seismic answer of the buildings" E SE MT 82 - 01 SG 1

## 3 Modeling A

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### 3.1 Characteristics of modeling

#### Grid of the foundation

The foundation in the shape of cross:

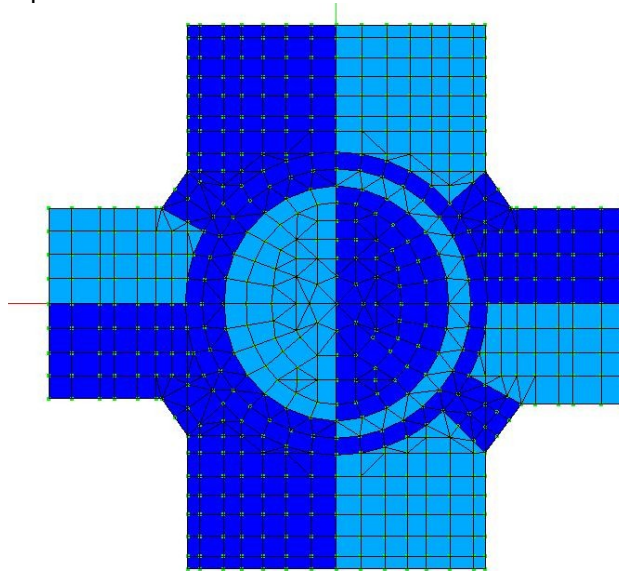


Figure 3.1-a: Grid of the foundation

#### Grid of the building

The model hollow 3D of the building consists of elements of plates:

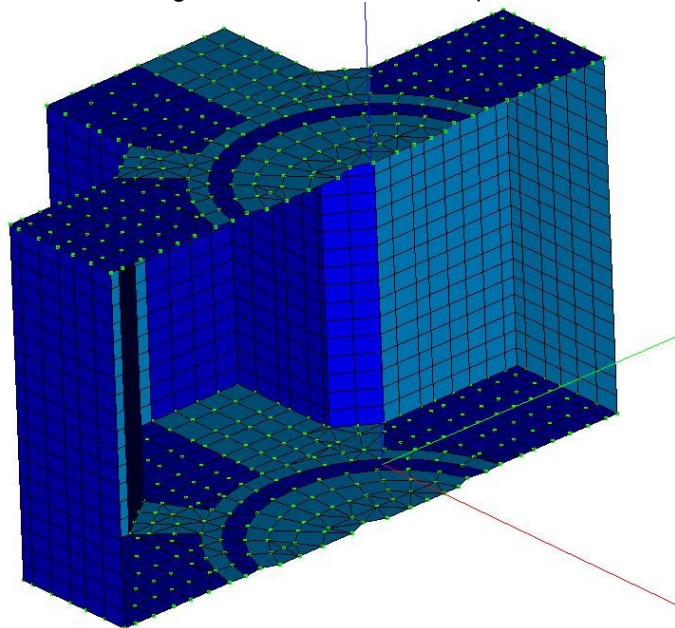


Figure 3.1-b: Representation out of cut of the grid of the building



Damping is initially regarded as modal damping taken equal to 7% for all the clean modes of the structure on embedded basis calculated in the seismic range for Eigen frequencies lower than 23 Hz .

An alternative modeling of damping consists in representing the damping of the structure in the form of a combination of Rayleigh with:  $\alpha=2.48E-3$  ,  $\beta=0.78$  .

## 3.2 Characteristics of the grid

The model is composed of 3,149 nodes and 3,432 elements plates.

## 3.3 Sizes tested and results

### 3.3.1 Withmortissement modal

The values of spectra tested are those obtained with the assumption of modal damping. The imposed requests are the accelerations given on [Figures 1.3] above.

### 3.3.2 Withmortissement of Rayleigh

The values of spectra tested are those obtained with the assumption of damping of Rayleigh the imposed requests are dieplacements resulting from a double integration of the accelerations given on [Figures 1.3] above.

## 4 Summary of the results

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The got results constitute tests of nonregression.